

## Standard Slab Bridge – General Instructions

### Makeup of Plans

#### 1. General Plan

See Section 3, *Bridge Design Details* manual. General notes have been placed on the Slab Reinforcement Details sheet.

#### 2. Deck Contour

See Section 4, *Bridge Design Details* manual.

#### 3. Foundation Plan

See Section 5, *Bridge Design Details* manual.

#### 4. Abutments and Bent Details

See Section 6 & 7, *Bridge Design Details* manual. Wingwalls on Standard Plan B0-1 will usually be satisfactory. Bent caps for spans 24' and less should be detailed in the usual manner (plan, elevation and section). The dropped portion should be squared off at the ends, 1'-0" minimum from edge of deck. Plan, elevation and sectional views are still desirable. For flush caps, the width of stirrups must be indicated.

#### 5. Deck Details

Plan views of both top and bottom slab reinforcement are required. Indicate length, total number and placement data for each class of main reinforcing bars. Also show the typical section, the camber diagram, main reinforcing bars and diagram for payment of concrete. A longitudinal deck sectional view is unnecessary.

#### 6. Slab Reinforcement Details

Insert this 'Standard' sheet as part of the plans for every slab bridge.

#### 7. Slab Hinge Details

Insert this 'Standard' sheet where a hinge is required. Detail joint seal data, 'A' bar size, and elastomeric bearing pad size. Use the steel hinge *only* when matching an existing one in a widening.

## 8. Piles

Insert the appropriate 'Standard' sheet. At locations where aesthetics are not important, allow the steel shell to be used full height by modifying the standard sheet with a note to that effect. The shell should extend 1" into the soffit.

## 9. Railing Details

Refer to book of Standard Plans or insert appropriate sheets.

## 10. Reference to book of Standard Plans

These sheets will always be required: A62-C 'Limits of Payment for Excavation and Backfill Bridge', B0-1 and B0-3 'Bridge Details'. Others should be referred to when required.

## 11. Log of Test Borings

Insert as usual.

### Basis of Design

#### 1. Design Method

Load factor design based on Bridge Design Specifications. See 'Slab Reinforcement Details' for general notes.

#### 2. Distribution of Wheel Loads

Distribution of wheel loads conform to Article 3.24.3.2 of the 1983 AASHTO.  $E = 4 + 0.06S$  (max = 7').

#### 3. Slab Thickness

The thickness of the slab is designed in accordance with Article 8.9.2 and Table 8.9.2 of the 1983 AASHTO.  $L/300$  was used as a maximum deflection criteria.

#### 4. Environmental Factor $Z$

170 kip/in.

#### 5. Span Length

Actual span lengths are shown for all except 'D' spans. If a span configuration of 18-24-18 is desired, the chart values for  $L = 24'$  should be used for all 3 spans. 'D' span values in the chart are

based on  $(0.75) L$  or 18' in this case. Criteria for intermediate span lengths may be interpolated from the values given in these standards.

## 6. Skew

The charts for superstructure design may be used for skews up to 50 degrees. A special design should be used when the skew angle exceeds 50 degrees. Piles may have to be added at the abutments to support the obtuse corners of the slab. See 'Support Design Data' and 'Typical Support Calculations' sheets.

## 7. End Diaphragm Abutment

Designers will determine the longitudinal force based upon Memo to Designers 5-2. To determine longitudinal force, divide total force by effective abutment width.

## 8. Hinges

Two types of hinges are available. The hinge at bent should be used on the shorter spans where dropped caps are normally required or on the longer spans where appearance is not a factor. The hinge at  $L/6$  is for use on spans without dropped caps and where it is desired to have the soffit line remain unbroken for aesthetic purposes.

## 9. Piles

The following limitations apply.

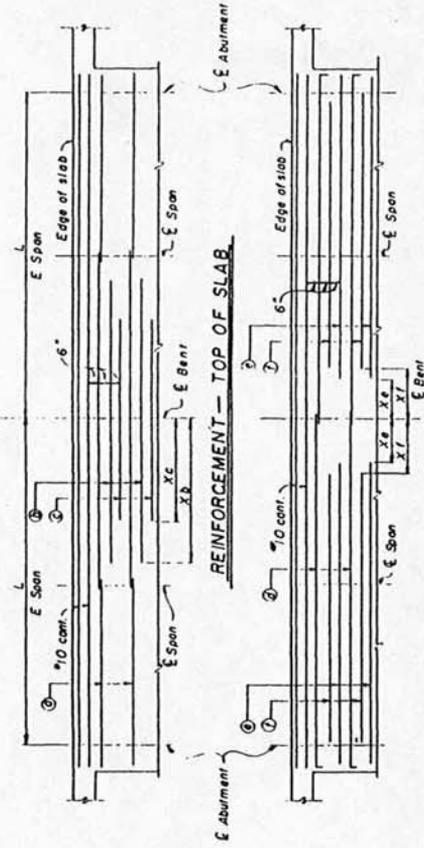
- a. Relatively uniform unsupported heights between 8 and 25 feet, before and after considering scour.
- b. Compact sandy soil or better and/or stiff clay or better.
- c. Expansion joints shall be located at approximately 250 feet on centers.
- d. Exceptions to the above will necessitate a design analysis.

## 10. Drainage

Scuppers or deck drains must be specially detailed when necessary.

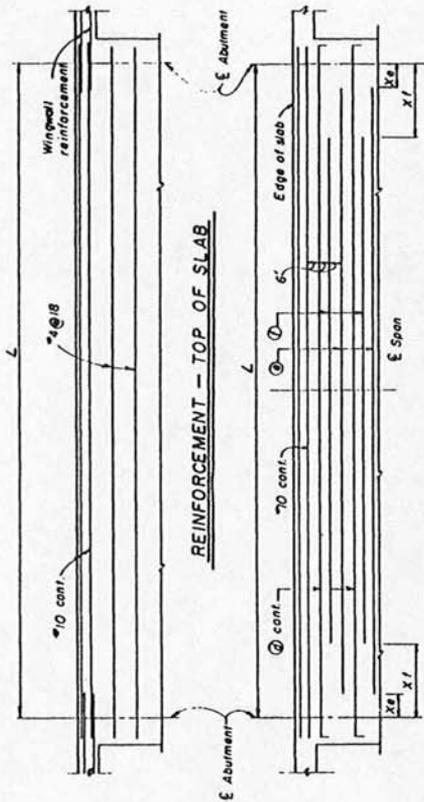
## 11. Quantities

Approximate slab quantities are given for one lineal foot of slab width. The reinforcement for caps and end diaphragms as well as the concrete extending outside the slab limits are not included in these charts.



REINFORCEMENT - TOP OF SLAB

REINFORCEMENT - BOTTOM OF SLAB



REINFORCEMENT - BOT. OF SLAB

REINFORCEMENT	L = Length of Span		20'		22'		24'		26'		30'		34'		36'		40'		44'	
	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing
Top of Slab	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"
Bottom of Slab	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"
Approximate Quantity	13.8		14.2		14.6		15.0		15.4		15.8		16.2		16.6		17.0		17.4	
Ultimate Deflection at Midspan	0.01		0.01		0.01		0.01		0.01		0.01		0.01		0.01		0.01		0.01	

REINFORCEMENT	L = Length of Span		20'		22'		24'		26'		30'		34'		36'		40'		44'	
	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing	Size	Spacing
Top of Slab	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"
Bottom of Slab	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"	7	18"
Approximate Quantity	13.8		14.2		14.6		15.0		15.4		15.8		16.2		16.6		17.0		17.4	
Ultimate Deflection at Midspan	0.01		0.01		0.01		0.01		0.01		0.01		0.01		0.01		0.01		0.01	

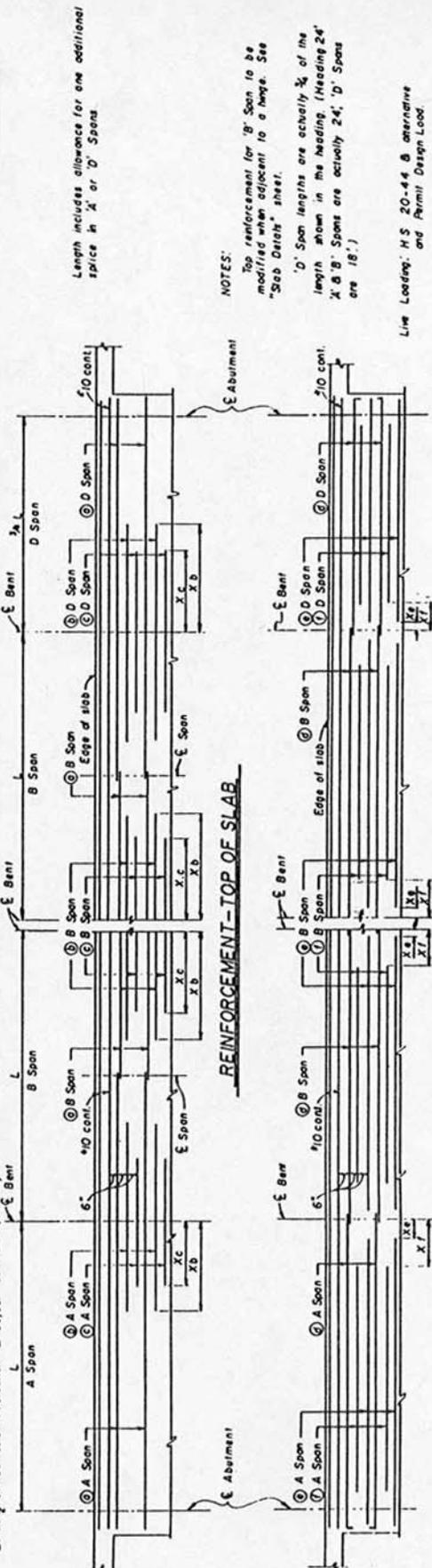
Ⓢ Add 1/2" for Corrosion Protection & adjust concrete quantity.  
 Live Loading: HS 20-44 and Alternative and Permit Design Load.

Ⓢ Add 1/2" for Corrosion Protection & adjust concrete quantity.  
 Live Loading: HS 20-44 and Alternative and Permit Design Load.

STANDARD SLAB BRIDGE  
 SLAB DETAILS - SINGLE & 2 SPAN



L = Length of Span Top of Slab	16'		18'		20'		22'		24'		26'		28'		30'		32'		34'		36'		40'		42'		44'			
	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B		
Length	16.0	18.0	20.0	22.0	24.0	26.0	28.0	30.0	32.0	34.0	36.0	40.0	42.0	44.0	16.0	18.0	20.0	22.0	24.0	26.0	28.0	30.0	32.0	34.0	36.0	40.0	42.0	44.0		
Area	1.00	1.33	1.78	2.33	2.99	3.78	4.69	5.72	6.87	8.14	9.53	11.04	12.67	14.42	1.00	1.33	1.78	2.33	2.99	3.78	4.69	5.72	6.87	8.14	9.53	11.04	12.67	14.42		
Weight	15.7	21.3	28.7	37.9	48.1	59.4	71.8	85.3	100.0	115.8	132.8	151.0	170.4	191.0	15.7	21.3	28.7	37.9	48.1	59.4	71.8	85.3	100.0	115.8	132.8	151.0	170.4	191.0	211.6	
Ultimate Deflection of Member $\Delta$	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	
Notes for Corrosion Protection $\delta$ adjust concrete quantity																														



NOTES:  
 Top reinforcement for 'B' Span to be modified when adjacent to a hinge. See "Slab Detail" sheet.  
 'D' Span lengths are actually  $\frac{3}{4}$  of the length shown in the heading. (Heading 24' 'A' & 'B' Spans are actually 24', 'D' Spans are 18'.)  
 Live Loading: H.S. 20-4.4 & alternative and Permit Design Load.

REINFORCEMENT-TOP OF SLAB  
 REINFORCEMENT-BOTTOM OF SLAB

STANDARD SLAB BRIDGE  
 SLAB DETAILS - MULTI SPAN





Support Type	LENGTH OF SPAN "L"														
	16'	18'	20'	22'	24'	26'	28'	30'	32'	34'	36'	38'	40'	42'	44'
Unfactored Loads	(1)	106	129	148	174	196	225	255	282	317	345	383	414	456	500
Slab Dead Load Reaction	(1)	3.04	3.68	4.24	4.97	5.57	6.44	7.12	8.06	9.07	10.96	11.84	13.04	14.29	15.28
(kip per foot width of slab includes 35 7/16" AC surfacing)	(1)	7.4	9.03	10.4	12.2	13.7	15.7	17.5	19.7	22.2	24.2	26.9	29.0	32.0	35.1
(kip per foot)	(1)	3.32	4.03	4.63	5.49	6.12	7.03	7.78	8.81	9.90	10.78	11.99	12.95	14.24	15.62
(kip per foot)	(1)	1.12	2.56	2.94	3.43	3.87	4.45	4.94	5.59	6.04	6.84	7.60	8.20	9.03	9.89
AC surfacing)	(1)	1.83	1.83	2.16	2.52	2.90	3.30	3.64	4.08	4.56	5.06	5.59	6.13	6.82	7.69
Live Load Reaction	(1)	40.60	41.42	42.04	42.50	43.03	43.70	45.00	46.50	47.80	49.00	50.1	51.1	52.0	52.9
(kip per foot)	(1)	470	499	526	554	575	595	610	623	634	643	651	658	664	669
(kip per foot)	(1)	40.3	41.6	42.7	43.6	44.5	45.0	45.2	45.8	46.3	46.6	46.8	47.0	47.1	47.2
(no impact)	(1)	48.2	51.4	53.5	56.4	58.6	60.4	61.9	63.1	64.0	64.9	65.5	66.2	66.7	67.2
(no impact)	(1)	42.0	42.7	43.2	43.6	43.3	42.8	42.0	41.6	41.0	40.6	40.3	40.0	39.7	39.4
Live Load Reaction	(1)	48.0	48.0	50.8	53.5	56.1	58.5	60.6	62.6	64.4	66.0	67.5	70.2	72.9	75.3
(kip per lane)	(1)	55.0	62.6	68.2	72.3	75.2	78.2	80.7	83.0	85.4	87.8	90.3	92.8	95.3	97.8
(kip per lane)	(1)	58.9	66.0	71.0	75.2	78.2	80.8	82.9	84.9	86.8	88.6	90.3	92.0	93.7	95.4
(kip per lane)	(1)	48.0	48.5	52.8	57.4	62.0	66.5	71.0	75.2	79.2	83.1	86.7	91.4	95.9	100.2
(kip per lane)	(1)	30	30	30	30	30	30	30	30	30	30	30	30	30	30
IMPACT	(1)	6.01	6.74	7.48	8.24	9.00	9.75	10.49	11.24	11.99	12.76	13.51	14.26	15.00	15.75
SPRINGS ONLY	(1)	6.40	7.21	8.00	8.89	9.59	10.40	11.21	12.00	12.79	13.59	14.40	15.20	15.99	16.81
Uniform Load	(1)	18.31	20.52	22.90	25.19	27.43	29.75	32.01	34.32	36.60	38.87	41.17	43.45	45.79	48.03
Use for railing, curbs, sidewalks (per span)	(1)	4.47	5.08	5.61	6.17	6.73	7.29	7.87	8.40	8.99	9.55	10.09	10.66	11.22	11.79
(kip per foot)	(1)	19.97	22.52	25.02	27.78	30.01	32.40	34.96	37.48	39.99	42.49	44.98	47.52	49.99	52.44
(kip per foot)	(1)	12.72	14.31	15.90	17.46	19.00	20.53	22.12	23.73	25.36	26.90	28.50	30.09	31.67	33.25
(kip per foot)	(1)	8.01	9.00	9.99	11.01	12.01	13.01	14.00	15.01	15.99	17.00	17.99	18.98	19.99	20.00
(kip per foot)	(1)	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88	1.88
(kip per foot)	(1)	5.1	4.7	4.5	4.1	3.9	—	—	—	—	—	—	—	—	—
(kip per foot)	(1)	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
(kip per foot)	(1)	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6	3.6
"b" (in inches)	(1)	16	18	20	22	24	26	28	30	32	34	36	38	40	42

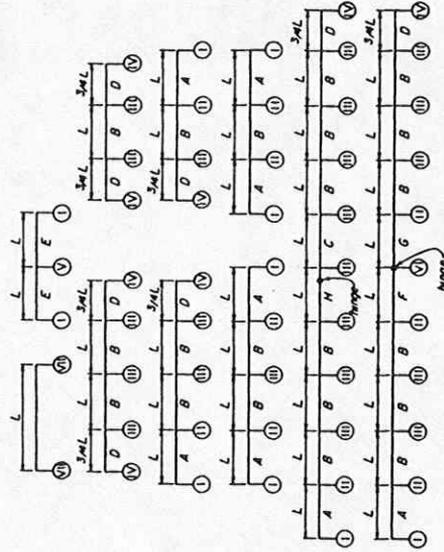
\* See Support Design Data No. 2 sheet.

Support Type	Assumed Total Depth of Section
(1) (M) (N)	7" plus 60"
(2) (U) (V)	24"
(3) (W) (X)	7"
(4) (Y) (Z)	7" plus 18"

LIVE LOADINGS 20-44 B ALTERNATIVE AND PERMIT DESIGN LOAD

Span Angle (in degrees)	"k"
20 - 26	34
27 - 32	36
33 - 38	38
39 - 44	40
45 - 50	42

REACTION COEFFICIENT "k"  
SKEWED BRIDGES  
To be used in pile calculation when bridge skew is 20° or more.

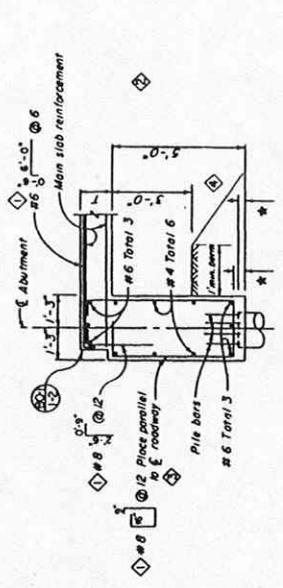
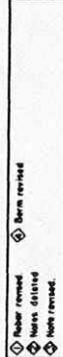


TYPICAL LAYOUTS

For determination of support types, Support Type shown thus: (1)  
Type of span shown thus: A

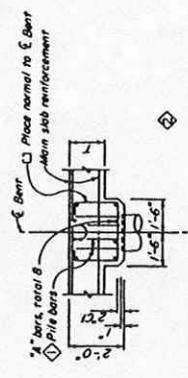
Support Type	LENGTH OF SPAN L														
	16'	18'	20'	22'	24'	26'	28'	30'	32'	34'	36'	38'	40'	42'	44'
45T Pile Spacing *	10'-6"	9'-3"	8'-6"	7'-9"	7'-3"	6'-9"	6'-3"	5'-9"	5'-6"	5'-0"	---	---	---	---	---
"A" bars (Slew 0°-20°)	#7	#7	#7	#7	#7	#6	#6	#6	#6	#6	#6	#6	#6	#6	#6
"A" bars (Slew 21°-35°)	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
"A" bars (Slew 36°-50°)	#9	#8	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
Stirrups	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'
70T Pile Spacing *	10'-6"	9'-3"	8'-6"	7'-9"	7'-3"	6'-9"	6'-3"	5'-9"	5'-6"	5'-0"	---	---	---	---	---
"A" bars (Slew 0°-20°)	#7	#7	#7	#7	#7	#6	#6	#6	#6	#6	#6	#6	#6	#6	#6
"A" bars (Slew 21°-35°)	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
"A" bars (Slew 36°-50°)	#9	#8	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
Stirrups	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'
45T Pile Spacing *	10'-6"	9'-3"	8'-6"	7'-9"	7'-3"	6'-9"	6'-3"	5'-9"	5'-6"	5'-0"	---	---	---	---	---
"A" bars (Slew 0°-20°)	#7	#7	#7	#7	#7	#6	#6	#6	#6	#6	#6	#6	#6	#6	#6
"A" bars (Slew 21°-35°)	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
"A" bars (Slew 36°-50°)	#9	#8	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
Stirrups	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'
70T Pile Spacing *	10'-6"	9'-3"	8'-6"	7'-9"	7'-3"	6'-9"	6'-3"	5'-9"	5'-6"	5'-0"	---	---	---	---	---
"A" bars (Slew 0°-20°)	#7	#7	#7	#7	#7	#6	#6	#6	#6	#6	#6	#6	#6	#6	#6
"A" bars (Slew 21°-35°)	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
"A" bars (Slew 36°-50°)	#9	#8	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
Stirrups	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'
45T Pile Spacing *	10'-6"	9'-3"	8'-6"	7'-9"	7'-3"	6'-9"	6'-3"	5'-9"	5'-6"	5'-0"	---	---	---	---	---
"A" bars (Slew 0°-20°)	#7	#7	#7	#7	#7	#6	#6	#6	#6	#6	#6	#6	#6	#6	#6
"A" bars (Slew 21°-35°)	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
"A" bars (Slew 36°-50°)	#9	#8	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
Stirrups	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'
70T Pile Spacing *	10'-6"	9'-3"	8'-6"	7'-9"	7'-3"	6'-9"	6'-3"	5'-9"	5'-6"	5'-0"	---	---	---	---	---
"A" bars (Slew 0°-20°)	#7	#7	#7	#7	#7	#6	#6	#6	#6	#6	#6	#6	#6	#6	#6
"A" bars (Slew 21°-35°)	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
"A" bars (Slew 36°-50°)	#9	#8	#8	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7	#7
Stirrups	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'	#5 @ 5" @ 50'

\* Note: Pile Spacing Above Are For No Slew Conditions, in (Normal)

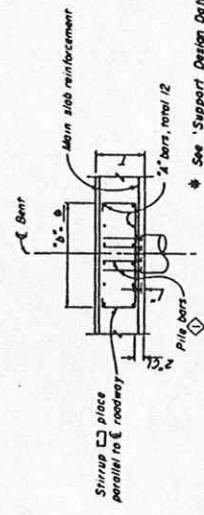


\* - See pile details

ABUTMENT SECTION



BENT CAP - SPANS 16'-24'



BENT CAP - SPANS 26'-44'

\* See 'Support Design Data No. 1' sheet.

STANDARD SLAB BRIDGE  
SUPPORT DESIGN DATA NO. 2

## STANDARD SLAB BRIDGE TYPICAL SUPPORT CALCULATIONS

## Example No. 1

Bridge width: 35'-6" (32' Roadway)

Length of Span: L = 32'

Type of Support: IV (Abutment)

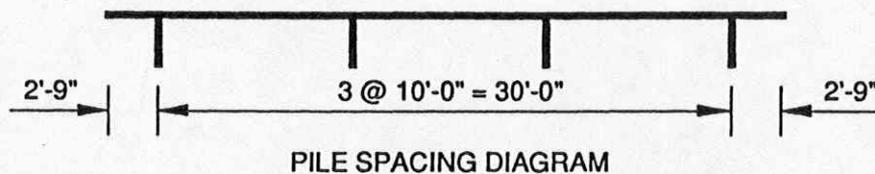
Allowable Pile value: 45 tons = 90 kips

No Skew

Pile calculations from table: L = 32' Support Type IV

90 kip Pile Spacing = 10'-3"

$$\begin{aligned} \text{No. of Piles required} &= \left[ \frac{\text{Bridge Width} - (2 \times \text{Edge Distance to Piles})}{10.25} \right] + 1 \\ &= \left[ \frac{35.5 - (2 \times 2)}{10.25} \right] + 1 \\ &= 3.07 + 1 \\ &= 4.07 \\ &\text{USE 4 PILES} \end{aligned}$$



## Example No. 2

Bridge width: 35'-6" (32' Roadway)

Length of Span: L = 42'

Type of Support: V (Bent)

Allowable Pile value: 70 tons = 140 kips

Skew Angle 46°

Pile calculations from table: L = 42' Support Type V

140 kip Pile Spacing = 5'-3"

$$\text{Skewed Pile Spacing} = \left( \frac{5.25}{\cos 46^\circ} \right) = 7.56$$

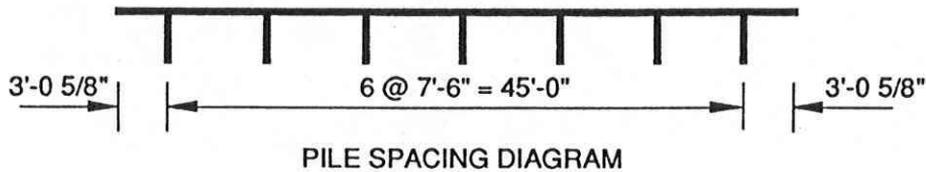
$$\begin{aligned} \text{Max. edge dist. to support} &= 0.4 \text{ Pile Spacing} \\ &= 0.4(7.56) \\ &= 3.00' \end{aligned}$$

$$\text{Length of Support} = \left( \frac{35.5}{\cos 46^\circ} \right) = 51.1$$

$$\begin{aligned} \text{No. of Piles required} &= \left[ \frac{\text{Length of Support} - (2 \times \text{Edge Dist. to Pile})}{7.56} \right] + 1 \\ &= \left[ \frac{51.1 - (2 \times 3)}{7.56} \right] + 1 \\ &= 6.97 \\ &\text{USE 7 PILES} \end{aligned}$$

Pile Spacing = 7'-6"

$$\text{Edge Distance to Exterior Pile} = \left[ \frac{51.1 - (6 \times 7.5)}{2} \right] = 3.05'$$



Bent Cap Reinforcement: "A" bars #7  
Stirrup #6 @ 8

### Example No. 3

Bridge width: 58'-3" ave (54'-9" ave Roadway)

Length of Span: L = 32'

Type of Support: II (Bent)

Allowable Pile value: 70 tons = 140 kips

No Skew

Pile calculations from table: L = 32' Support Type II

140 kip Pile Spacing = 8'-0"

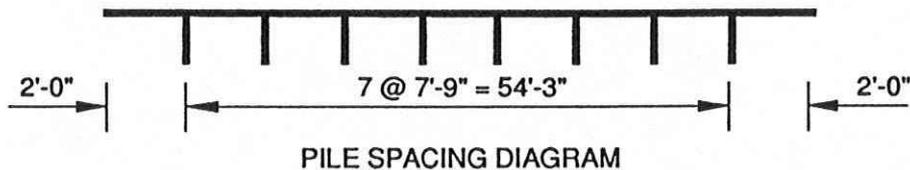
$$\text{No. of Piles required} = \left[ \frac{58.25 - (2 \times 2)}{8} \right] + 1$$

$$= 7.78$$

USE 8 PILES

Pile Spacing = 7'-9"

$$\text{Edge Distance to Centerline Exterior Pile} = \left[ \frac{58.25 - (7 \times 7.75)}{2} \right] = 2.0'$$



Bent Cap Reinforcement: "A" bars #8  
Stirrup #6 @ 7

Example No. 4

Bridge width: 35'-6" (32' Roadway)

Length of Span: L = 30'

Type of Support: I (Abutment)

Allowable Pile value: 45 tons = 90 kips

Skew Angle: 39°

Pile calculations from table: L = 30' Support Type I

90 kip Pile Spacing = 9'-3"

$$\text{Skewed Pile Spacing} = \left( \frac{9.25}{\cos 39^\circ} \right) = 11.9'$$

$$\text{Length of Support} = \left( \frac{35.5}{\cos 39^\circ} \right) = 45.68'$$

Maximum edge distance to support = 0.4 Pile Spacing, try 3'

$$\begin{aligned} \text{No. of Piles required} &= \left[ \frac{45.68 - (2 \times 3)}{11.9} \right] + 1 \\ &= 4.33 \end{aligned}$$

USE 5 PILES

Pile Spacing = 10'-0"

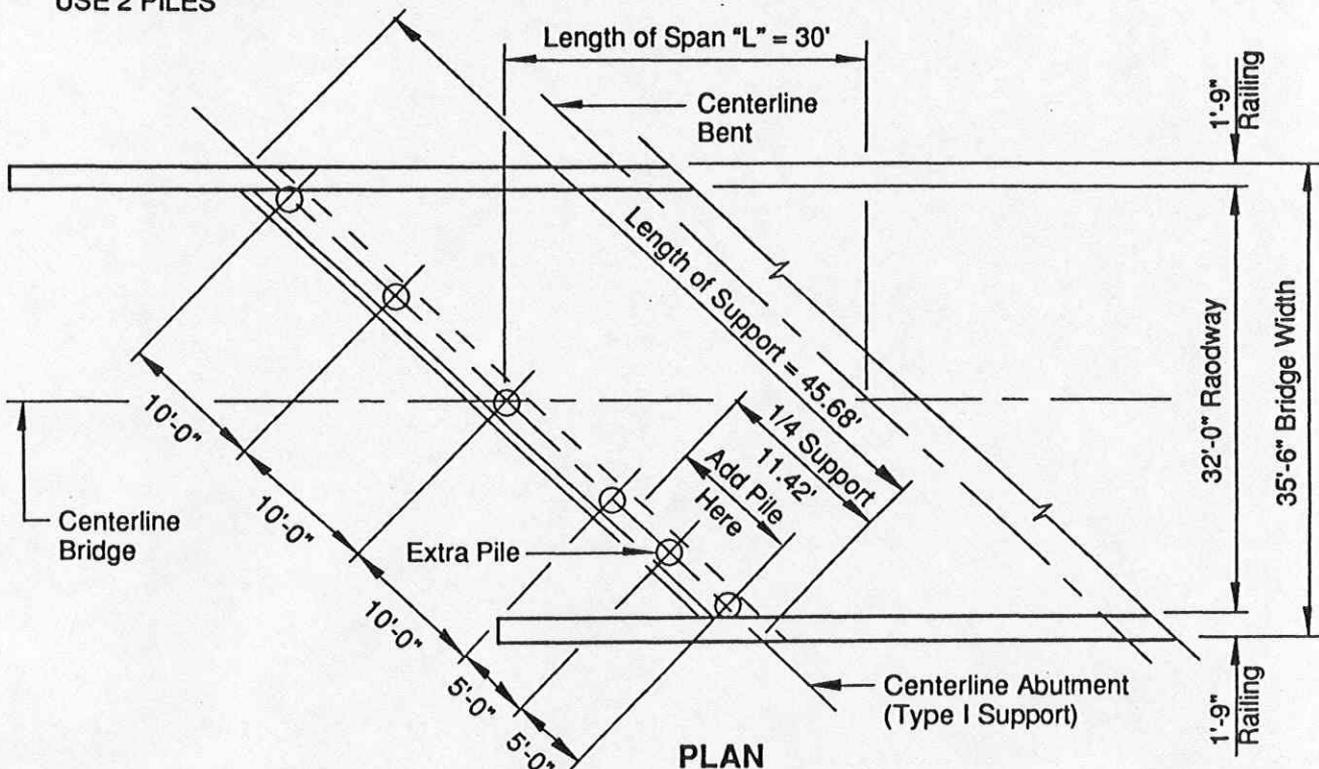
$$\text{Edge Distance to Centerline Exterior Pile} = \left[ \frac{45.68 - (4 \times 10)}{2} \right] = 2.84' = 2'-10 \frac{1}{8}"$$

Skew Angle: 39°

Reaction Coefficient: "K" = 0.40

Number of Piles required under end 1/4 support at Obtuse Corner  $5 \times 0.4 = 2$

USE 2 PILES



## Example No. 5

Bridge width: 55'-6" (52' Roadway)

Length of Span:  $L = 24'$

Type of Support: VI (Expansion Bent)

Allowable Pile value: 70 tons = 140 kips

Skew Angle:  $18^\circ 30'$

Pile calculations from table:  $L = 24'$  Support Type VI

90 kip Pile Spacing = 9'-0"

140 kip Pile Spacing =  $\left(\frac{9 \times 140}{90}\right) = 14' > 10'-6''$  max. cap span from chart

Skewed Pile Spacing =  $\left(\frac{10.5}{\cos 18.5^\circ}\right) = 11.07'$

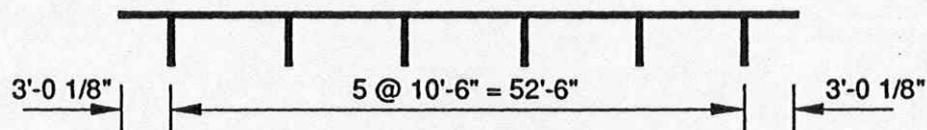
Length of Support =  $\left(\frac{55.5}{\cos 18.5^\circ}\right) = 58.52'$

Minimum edge distance for Pile into a drop cap =  $\left(\frac{2.5}{\cos 18.5^\circ}\right) = 2.64'$

No. of Piles required =  $\left[\frac{58.52 - (2 \times 2.64)}{11.07}\right] + 1$

= 5.80

USE 6 PILES

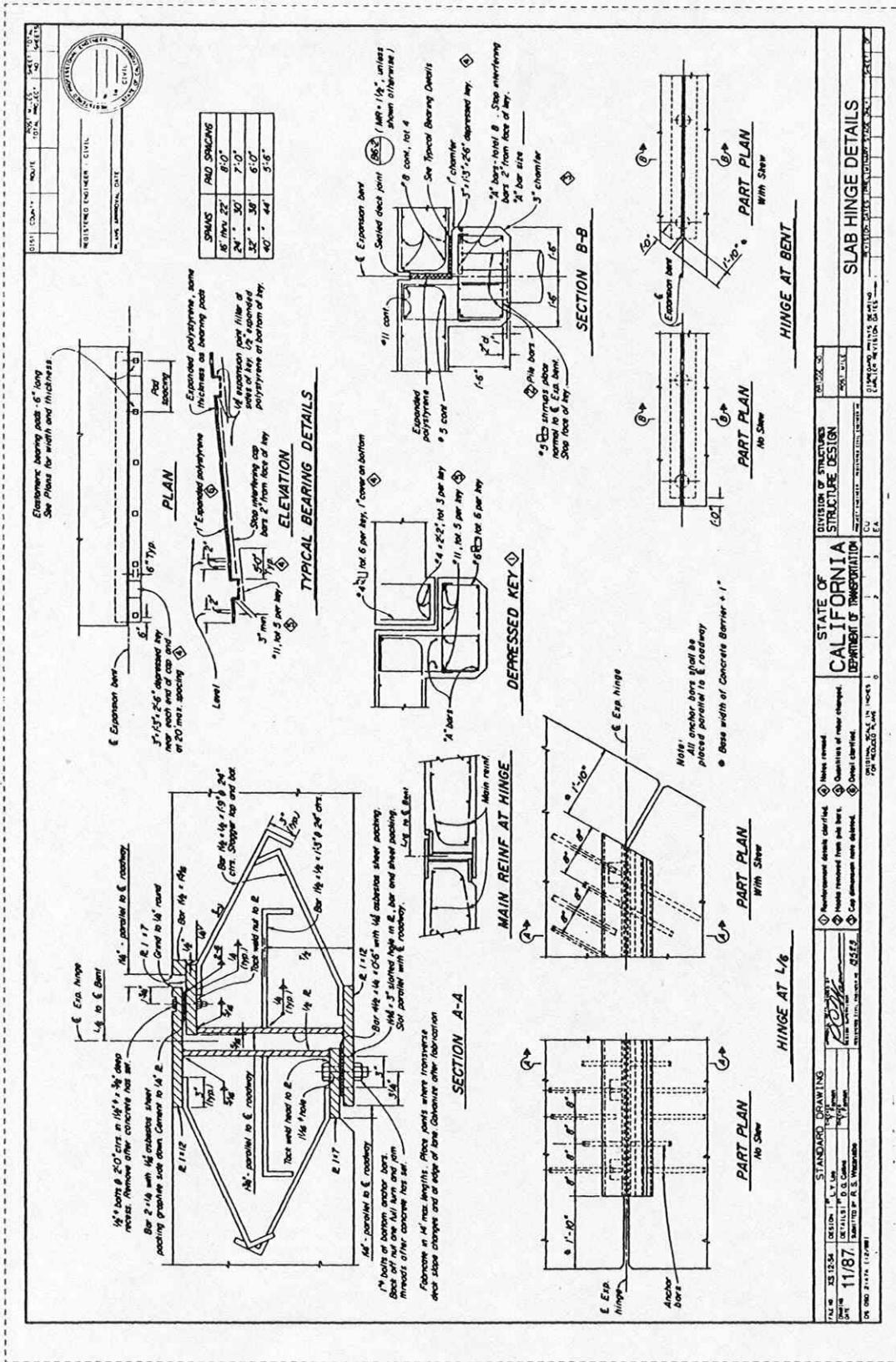


Bent Cap Reinforcement from table: Support Type VI

140 kip Pile Spacing @ 10'-6", "A" Bars #10

Stirrup #6 @ 6

One pile per bent could be saved by designing a larger cap.



Note to Designer: 1'-10" edge clearance of metal assembly is for Concrete Barrier Type 25. Change to 1'-6" when Concrete Barrier Type 27 is used to provide 1" clearance. Specify bearing thickness and width, Joint Seal MR and Waterstop on the detail plans. Use of Steel Hinge Detail at L/6 should be avoided as much as possible.

DESIGN: BRIDGE DESIGN SPECIFICATIONS  
LOAD FACTOR DESIGN

DESIGN: (1) FOR ADJUST TO WITH HEIGHTS AND FREQUENCIES BY CALTRANS  
DEAD LOAD: INCLUDES 25 psf for future wearing surface.  
LIVE LOADING: HS20-44 and alternative and permit design load.  
REINFORCED CONCRETE:  $f_c = 40,000$  psi  
 $f_s = 220,000$  psi

**LONGITUDINAL SECTION**

**EDGE OF SLAB DETAIL**

**BAR CHAIR DETAIL**

**TOP SLAB REINFORCEMENT AT BENT**

**FLUSH CAP**

**DROPPED CAP**

**BOTTOM SLAB REINFORCEMENT AT BENT**

**REINFORCEMENT NOTES:**

- Splices in top main bars to be located near center of span.
- Splices in bottom main bars to be located near bent.
- Spacing of all transverse bars to be measured along  $\epsilon$  roadway.
- Show 0' to 20' - Place all transverse bars parallel to bent.
- Show over 20' - Place transverse slab bars perpendicular to bent.
- Show over 20' -  $\epsilon$  bridge. See details at right and below.

Bar Size	14	15	16	17	18	19	20	21
All bars, except top bars	23"	28"	34"	39"	45"	50"	56"	61"
Top bars in spans over 24'	23"	28"	34"	39"	45"	50"	56"	61"

**GENERAL NOTES:**

DESIGN: BRIDGE DESIGN SPECIFICATIONS  
LOAD FACTOR DESIGN

DESIGN: (1) FOR ADJUST TO WITH HEIGHTS AND FREQUENCIES BY CALTRANS  
DEAD LOAD: INCLUDES 25 psf for future wearing surface.  
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REINFORCED CONCRETE:  $f_c = 40,000$  psi  
 $f_s = 220,000$  psi

STATE OF CALIFORNIA  
DIVISION OF STRUCTURES  
STRUCTURE DESIGN

STANDARD DRAWING  
8/86  
DESIGNED BY: R. J. WILLIAMS  
CHECKED BY: R. J. WILLIAMS

SLAB REINFORCEMENT DETAILS

Note to Designer: Clearances shown for top reinforcement on longitudinal sections should be increased for Marine Environment.

